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Some implications from the in-depth study and simulation modelling of road departure crashes on bends on rural roads

## Clear Zones



## Clear Zone Correction for Curves

- Crash histories indicate a need
- A specific site investigation shows a definitive crash potential that could be significantly reduced by increasing the clear zone width, and
- Such increases are cost-effective.

Table 4.2: Curve correction factors

| Radius (m) | Design speed (km/h) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 60 | 70 | 80 | 90 | 100 | 110 |  |
| 900 | 1.1 | 1.1 | 1.1 | 1.2 | 1.2 | 1.2 |  |
| 700 | 1.1 | 1.1 | 1.2 | 1.2 | 1.2 | 1.3 |  |
| 600 | 1.1 | 1.2 | 1.2 | 1.2 | 1.3 | 1.4 |  |
| 500 | 1.1 | 1.2 | 1.2 | 1.3 | 1.3 | 1.4 |  |
| 450 | 1.2 | 1.2 | 1.3 | 1.3 | 1.4 | 1.5 |  |
| 400 | 1.2 | 1.2 | 1.3 | 1.3 | 1.4 | - |  |
| 350 | 1.2 | 1.2 | 1.3 | 1.4 | 1.5 | - |  |
| 300 | 1.2 | 1.3 | 1.4 | 1.5 | 1.5 | - |  |
| 250 | 1.3 | 1.3 | 1.4 | 1.5 | - | - |  |
| 200 | 1.3 | 1.4 | 1.5 | - | - | - |  |
| 150 | 1.4 | 1.5 | - | - | - | - |  |
| 100 | 1.5 | - | - | - | - | - |  |

Source: AASHTO (2006).

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## Some terminology



## Superelevation



Figure 7.8: Typical superelevation development profile on two lane roads (tangent to transition curve to circular curve

## Background

- The practice of using clear zones in lieu of barrier protection needs to be challenged
- 10-20\% of errant vehicles will exceed the 10.5 m clear zone
- Even if $\geq 10.5 \mathrm{~m}$ is achieved, surface often contains trip hazards
- this is exacerbated on bends (due to superelevation etc)
- Previous simulation modelling work at CASR
- Departures on straights
- Influence of wide medians and centreline barriers
- Interplay between clear zones and barriers from a Safe Systems perspective
- Only the shallow angle drift off departures are catered for


## Crashes associated with curves on rural roads

- Curves are over-represented in rural road crashes
- 2001 to 2005 (Tziotis et al 2005):
- $27 \%$ of all injury crashes
- $52 \%$ of road departure crashes (run off road)
- Issue for young drivers (Clark et al 2010)
- Many factors associated with crash rates:
- Radius
- Super-elevation
- Grade
- Transitions
- Signs and delineation


## CASR In-depth crash database

- Several different studies in rural areas
- Criterion of ambulance call out / transport to hospital
- Bias towards daytime crashes during business hours

| Crash severity | Number | Percentage |
| :--- | :--- | :--- |
| PDO | 4 | $6.3 \%$ |
| Doctor | 3 | $4.7 \%$ |
| Treated | 20 | $31.3 \%$ |
| Admitted | 18 | $28.1 \%$ |
| Fatal | 19 | $29.7 \%$ |
| Total | 64 | $100.0 \%$ |

## Characteristics of the sample

| Speed zone $(\mathrm{km} / \mathrm{h})$ | Number | Percentage |
| :--- | :--- | :--- |
| 80 | 7 | $10.9 \%$ |
| 100 | 44 | $68.8 \%$ |
| 110 | 13 | $20.3 \%$ |
| Total | 64 | $100.0 \%$ |


| Rollover | Number | Percentage |
| :--- | :--- | :--- |
| No | 37 | $57.8 \%$ |
| Yes | 27 | $42.2 \%$ |
| Total | 64 | $100.0 \%$ |


| Lighting | Number | Percentage |
| :--- | :--- | :--- |
| Day | 49 | $76.6 \%$ |
| Night | 15 | $23.4 \%$ |
| Total | 64 | $100.0 \%$ |

## Road Departure Types



## Departure Characteristics

| Departure direction |  | Curve radius |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 0-199 | 200-399 | 400+ | Total |
| Left | Left bend | 6 | 1 | 2 | 9 |
|  | Right bend | 15 | 7 | 4 | 26 |
| Right | Left bend | 5 | 2 | 1 | 8 |
|  | Right bend | 12 | 6 | 3 | (21) |
| Total |  | 38 | 16 | 10 | 64 |



# Initiation of departure and actual point of departure relative to apex 

|  |  | Departure initation |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |
| Departure | Before <br> curve | Before apex | After apex | Unknown | Total |  |
| Before apex | 1 | 6 | 0 | 0 | 7 |  |
| Atter apex | 1 | 8 | 13 | 2 | 24 |  |
| After bend | 0 | 5 | 26 | 0 | 31 |  |
| Unknown | 0 | 0 | 0 | 2 | 2 |  |
| Total | 2 | 19 | 39 | 4 | 64 |  |

Cumulative distribution of curve radius for road departure crashes on curves ( $n=64$ )


## Run off road crashes on curves by advisory speed and curve radius

Curve radius

| Advisory speed (km/h) | $0-199$ | $200-399$ | $400+$ | Total |
| :--- | :---: | :---: | :---: | :---: |
| 25 | 2 | 0 | 0 | 2 |
| 45 | 2 | 0 | 0 | 2 |
| 55 | 7 | 0 | 0 | 7 |
| 60 | 1 | 0 | 0 | 1 |
| 65 | 7 | 0 | 0 | 7 |
| 70 | 1 | 0 | 0 | 1 |
| 75 | 1 | 4 | 1 | 6 |
| 80 | 2 | 0 | 0 | 2 |
| 85 | 0 | 3 | 1 | 4 |
| 95 | 1 | 0 | 0 | 1 |
| Curve Advisory Sign | 1 | 2 | 0 | 3 |
| None | 13 | 7 | 8 | 28 |
| Total | 38 | 16 | 10 | 64 |

## Departure Speed vs Advisory Speed (n=16)



## Departure Speed by Curve Radius



# Friction demand if vehicles had to negotiate the bend at their chosen entry speed (proxy was the estimated departure speed) 

| Friction demand | Number | Percentage |
| :--- | :---: | :--- |
| $0-0.19$ | 7 | $20.0 \%$ |
| $0.2-0.39$ | 16 | $45.7 \%$ |
| $0.4-0.59$ | 7 | $20.0 \%$ |
| $0.6-0.79$ | 3 | $8.6 \%$ |
| $0.8-0.99$ | 2 | $5.7 \%$ |
| Total | 35 | $100.0 \%$ |

## Longitudinal and lateral displacement by curve radius (an outlier removed for clarity)



## Cumulative distributions of lateral displacements by curve radius category



## Simulated Cases

| Case | Type | Bend <br> direction | Radius <br> (metres) | Departure <br> dngle <br> (degrees) | Lateral <br> displacement <br> (metres) | Speed <br> one <br> (km/h) | Rollover | Severity |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| R011 | Drift off | Left | 44 | 1 | 2.3 | 100 | No | Fatal |
| R037 | Drift off | Right | 107 | 16 | 3.9 | 100 | No | Admitted |
| R202 | Drift off | Right | 466 | 5 | 5.3 | 110 | No | Admitted |
| C054 | Single yaw | Right | 61 | 13 | 2.8 | 100 | No | Treated |
| R151 | Single yaw | Left | 74 | 16 | 3.6 | 80 | No | Treated |
| R238 | Single yaw | Right | 373 | 19 | 4.5 | 100 | No | Admitted |
| C031 | Double yaw | Right | 318 | 16 | 11.2 | 100 | Yes | Admitted |
| R106 | Double yaw | Right | 133 | 17 | 2.9 | 100 | No | Fatal |
| R135 | Double yaw | Left | 73 | 25 | 9.1 | 100 | Yes | Admitted |

## Lateral Displacement - Steering input only



## Lateral displacement - Braking only



## Barrier Normal Velocity

- Proxy for injury threshold
- $30 \mathrm{~km} / \mathrm{h}$ for side impacts



## Barrier normal velocity relative to the lateral offset of the barrier - steering input only



## Barrier normal velocity relative to the lateral offset of the barrier - braking input only



## Conclusions

- 1om clear zones on curves assist with drift off departures
- Yawing departures not well catered for
- Better off having barriers as close as practicable to edge of road
- Rollovers are a major issue
- Majority of departures beyond apex of curve
- Excessive speed (in terms of exceeding friction demand) did not seem to be a dominant factor in the sample of crashes
- Protection on inside of curves also necessary


## Implications

- Providing a clear zone in lieu of barrier protection is not recommended
- Better off having narrower clear zones and using barriers
- Barrier protection needed on inside of curves as well
- Barrier protection length needs consideration on the exit tangents and beyond


## Limitations

- Barrier performance under varying impact configurations
- Unsure of proportion of road departure crashes that involve yawing or drift off
- Assumption of level terrain from edge of road
- Representativeness of sample
- Ideally would look at horizontal curve sub categories in more detail as well (simple, reverse, compound, spiral)


## Ongoing Work

- Rollover and tripping mechanisms in clear zones
- What level of imperfections can be tolerated in a clear zone?
- Terrain
- Cross fall and slopes
- Hinge points and drop offs


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- The Centre for Automotive Safety Research receives supporting funding from the South Australian Department for Planning, Transport and Infrastructure and the Motor Accident Commission
- The views expressed in this report are those of the presenter and do not necessarily represent those of the University of Adelaide or the funding organisations
- For further information go to:


## www.casr.adelaide.edu.au

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# mac 

Motor Accident Commission

Table 4.1: Clear zone distances from edge of through travelled way

| Design speed (km/h) | Design ADT | Clear zone width (m) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Fill batter |  |  | Cut batter |  |  |
|  |  | 6:1 to flat | 4:1 to 5:1 | 3:1 and steeper ${ }^{(2)}$ | 6:1 to flat | 4:1 to 5:1 | 3:1 and steeper ${ }^{(2)}$ |
| $\leq 60$ | < 750 | 3.0 | 3.0 | (2) | 3.0 | 3.0 | 3.0 |
|  | 750-1500 | 3.5 | 4.5 | (2) | 3.5 | 3.5 | 3.5 |
|  | 1501-6000 | 4.5 | 5.0 | (2) | 4.5 | 4.5 | 4.5 |
|  | > 6000 | 5.0 | 5.5 | (2) | 5.0 | 5.0 | 5.0 |
| 70-80 | < 750 | 3.5 | 4.5 | (2) | 3.5 | 3.0 | 3.0 |
|  | 750-1500 | 5.0 | 6.0 | (2) | 5.0 | 4.5 | 3.5 |
|  | 1501-6000 | 5.5 | 8.0 | (2) | 5.5 | 5.0 | 4.5 |
|  | > 6000 | 6.5 | 8.5 | (2) | 6.5 | 6.0 | 5.0 |
| 90 | < 750 | 4.5 | 5.5 | (2) | 3.5 | 3.5 | 3.0 |
|  | 750-1500 | 5.5 | 7.5 | (2) | 5.5 | 5.0 | 3.5 |
|  | 1501-6000 | 6.5 | 9.0 | (2) | 6.5 | 5.5 | 5.0 |
|  | > 6000 | 7.5 | $10.0{ }^{(1)}$ | (2) | 7.5 | 6.5 | 5.5 |
| 100 | < 750 | 5.5 | 7.5 | (2) | 5.0 | 4.5 | 3.5 |
|  | 750-1500 | 7.5 | $10.0{ }^{(1)}$ | (2) | 6.5 | 5.5 | 4.5 |
|  | 1501-6000 | 9.0 | $12.0^{(1)}$ | (2) | 8.0 | 6.5 | 5.5 |
|  | > 6000 | $10.0{ }^{(1)}$ | $13.5{ }^{(1)}$ | (2) | 8.5 | 8.0 | 6.5 |
| 110 | < 750 | 6.0 | 8.0 | (2) | 5.0 | 5.0 | 3.5 |
|  | 750-1500 | 8.0 | $11.0^{(1)}$ | (2) | 6.5 | 6.0 | 5.0 |
|  | 1501-6000 | $10.0{ }^{(1)}$ | $13.0{ }^{(1)}$ | (2) | 8.5 | 7.5 | 6.0 |
|  | > 6000 | $10.5{ }^{(1)}$ | $14.0{ }^{(1)}$ | (2) | 9.0 | 9.0 | 7.5 |

1. Where a site specific investigation indicates a high probability of continuing crashes, or such occurrences are indicated by crash history, the designer may provide clear zone distances greater than the clear zone shown in Table 4.1. A jurisdiction may limit clear zones to 9 m for practicality and to provide a consistent roadway template if previous experience with similar projects or designs indicates satisfactory performance.
2. Since recovery is less likely on the unshielded, traversable $3: 1$ slopes, fixed objects should not be present in the vicinity of the toe of these slopes. Recovery of high-speed vehicles that encroach beyond the edge of the shoulder may be expected to occur beyond the toe of the slope. Determination of the recovery area at the toe of the slope should take into consideration available road reservation, environmental concerns, economic factors, safety needs, and crash histories. Also, the distance between the edge of the travelled lane and the beginning of the $3: 1$ slope should influence the recovery area provided at the toe of the slope. While the application may be limited by several factors, the fill slope parameters which may enter into determining a maximum desirable recovery area are illustrated in Figure 4.4

Notes:
The design ADT in the table is the average daily traffic volume in both directions and in all lanes, other than for divided roads where it is the total traffic in all lanes in ne direction.
Where the road is curved the values in Table 4.1 should be adjusted by the curve correction factors in Table 4.2
The RTA New South Wales uses a similar approach based on a hazard corridor and with curve adjustments included rather than ADT (Appendix C). For the same situation the RTA method results in greater clear zones than those shown in Table 4.1.
Source: Adapted from AASHTO (2006)

## Kloeden and McLean 1999



## Departure Speeds vs Design Speeds




[^0]:    Figure 4.5: Clear zone transition on approach to horizontal curves

